

# Graph for estimating field-scale hydraulic conductivity sampling requirements

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Buckland, G. D. 1988. Graph for estimating field-scale hydraulic conductivity sampling requirements. *Can. Agric. Eng.* 30: 323–324. A graph for estimating the number of saturated hydraulic conductivity measurements required in field studies is developed and described. The graph assumes hydraulic conductivity follows a log-normal distribution. Approximate confidence intervals (95%) around a geometric mean hydraulic conductivity can also be easily determined. Use of the graph does not require knowledge of statistics.

## INTRODUCTION

The saturated hydraulic conductivity ( $K_s$ ) of a soil is important in soil-water studies related to irrigation and drainage. Obtaining reliable estimates of a mean  $K_s$  is difficult because  $K_s$  is a highly variable property (Warrick and Nielsen 1980) with the degree of variability depending upon soil type (Willardson and Hurst 1965; Topp et al. 1980). Because the variability in  $K_s$  is soil dependent, the number of  $K_s$  determinations should be based on the variability in  $K_s$  encountered in the field.

Dylla and Guitjens (1970) used statistical procedures to develop a graph to estimate sampling requirements. Their work was based on the assumption that  $K_s$  follows a normal distribution. However, it has been shown that  $K_s$  follows a log-normal distribution (Willardson and Hurst 1965; Nielsen et al. 1973; Lee et al. 1985); the appropriate mean  $K_s$  is therefore the geometric mean (Bouwer 1969). Use of an arithmetic mean (normal distribution) results in an overestimate of the mean  $K_s$ .

The purpose of this work was to develop a graph, based on a log-normal distribution, which is simple and suitable for estimating  $K_s$  sampling requirements in the field.

## GRAPH DEVELOPMENT

The confidence interval around a geometric mean hydraulic conductivity ( $\bar{K}_s$ ) is described by

$$CI = \overline{\log K_s} \pm t \left( \frac{SDL^2}{n} \right)^{1/2} \quad (1)$$

where  $CI$  = confidence interval for log transformed data;  $\log K_s$  = mean of log transformed hydraulic conductivity,  $K_s$ ;  $t$  = Student's  $t$ , associated with  $n - 1$  degrees of freedom;  $SDL$  = standard deviation of log-transformed data; and  $n$  = number of  $K_s$  determinations. Use of either a log or natural log transformation is a matter of preference; either will result in identical geometric means and confidence intervals on data which has been transformed back to the original units.

Sample size requirements can be estimated using

$$n = t^2 SDL^2 / (\log D)^2 \quad (2)$$

where  $D$  is the desired accuracy. For log-normally distributed means  $D$  is the half width of the desired confidence interval (Steel and Torrie 1980) when expressed in terms of transformed (i.e., logarithmic) data. When expressed in terms of untransformed (i.e., nonlogarithmic) data,  $D$  takes the form of a multiplication/division factor (Topp et al. 1980).

As an example, assume  $\bar{K}_s$  was determined to be  $1.0 \text{ m d}^{-1}$  from 15 auger-hole tests. The calculated  $SDL$  was 0.4. The 95%  $CI$  by Eq. 1 is

$$CI = 0.000 \pm 2.145 \left( \frac{0.4^2}{15} \right)^{1/2} \\ = -0.222 \text{ to } 0.222$$

which, when untransformed

$$= 0.60 \text{ to } 1.67 \text{ m d}^{-1}$$

The desired accuracy ( $D$ ) as a multiplier/divisor factor is 1.0/0.60 and  $1.0 \times 1.67$  or 1.67. Substituting  $\log(1.67)$  into Eq. 2 for  $D$  yields:

$$n = (2.145)^2 (0.4)^2 / (0.222)^2 \\ = 15$$

which is the same number of predicted samples as samples which were taken. Thus, in the graphical solution to Eq. 2, as presented in Fig. 1,  $D$  is a multiplication/division factor relative to  $\bar{K}_s$ .

## GRAPH USE

To illustrate the use of the graph the following example is given. Auger-hole  $K_s$  tests are to be conducted in an area to assist with the determination of subsurface drain spacing. In this case the number of auger-hole  $K_s$  tests ( $n$ ) to produce an accuracy of 1.5 around the mean is required.

Five  $K_s$  determinations were conducted on the first day (Table I).  $\bar{K}_s$  and  $SDL$  were calculated to be  $0.13 \text{ m d}^{-1}$  and 0.20, respectively (Table I). Using the graph, select a desired accuracy of 1.5, move to an  $SDL$  of 0.2, and read across to  $n = 8$ . Therefore, at least three more determinations are required to obtain the desired accuracy. Alternatively, with  $n = 5$  and  $SDL = 0.2$ , read a desired accuracy of about 1.8. The 95%  $CI$  is  $0.13/1.8$  to  $0.13 \times 1.8$  or about  $0.07\text{--}0.23 \text{ m d}^{-1}$ . Five addi-

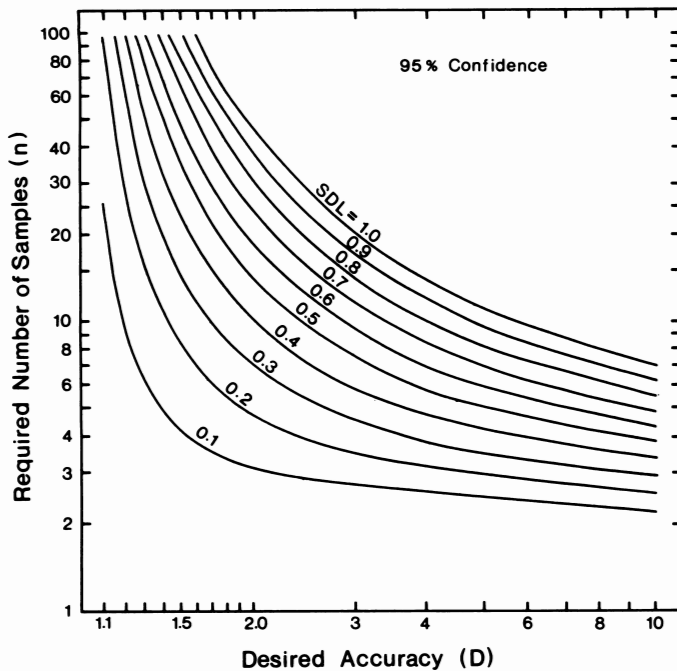


Figure 1. Graph for estimating hydraulic conductivity sample requirements.

tional  $K_s$  tests are conducted on the second day (Table I). Recalculation of  $\bar{K}_s$  and  $SDL$  yield 0.13 and 0.24, respectively. For  $n = 10$ , the desired accuracy is 1.5 (from Fig. 1); hence, a sufficient number of determinations has been made. The 95% CI is  $0.13/1.5 - 0.13 \times 1.5$  or about 0.09–0.20  $m d^{-1}$ .

#### OTHER CONSIDERATIONS

The  $SDL$  of Fig. 1 is independent of the units in which  $K_s$  is expressed (e.g.:  $m d^{-1}$  or  $m s^{-1}$ ). If a natural log (ln) transformation is used, the  $SDL$  of Fig. 1 can be multiplied by 2.3 to obtain the appropriate ln  $SDL$  (i.e., a log  $SDL$  of 0.1 corresponds to a ln  $SDL$  of 0.23).

Table I. Hydraulic conductivity determinations used in the example

$n$	$K_s$ ( $m d^{-1}$ )	$\log K_s$	$\bar{K}_s$	
			of cumulative samples ( $m d^{-1}$ )	$SDL$ of cumulative samples
1	0.10	-1.00	-	-
2	0.24	-0.62	-	-
3	0.08	-1.10	0.12	0.25
4	0.12	-0.92	0.12	0.21
5	0.19	-0.72	0.13	0.20
6	0.25	-0.60	0.15	0.21
7	0.05	-1.30	0.13	0.26
8	0.07	-1.15	0.12	0.26
9	0.13	-0.89	0.12	0.24
10	0.21	-0.68	0.13	0.24

At least three samples are necessary to obtain an initial estimate of  $SDL$ . If the field is heterogeneous, stratified random sampling (Petersen and Calvin 1986) should be used to delineate areas of different  $K_s$ , corresponding to different soils. This will result in a lower  $SDL$  for each soil type and thus a smaller  $n$  requirement for each soil type, but not necessarily a smaller  $n$  for the field.

Selection of  $D$  depends upon the intended use. For subsurface drainage studies Bouwer and Jackson (1974) suggest a  $\bar{K}_s$  within 30% ( $D = 1.3$ ) of the true field mean is reasonable. For variable soils, however, use of high accuracy (i.e., low  $D$ ) may result in impractical or prohibitive sample requirements. For example, if  $SDL = 0.4$  and  $D = 1.3$ , then  $n = 50$ . If  $D$  is increased to 1.5, however, then  $n$  decreases to 23. It therefore would require about one-half the time to determine  $\bar{K}_s$  using  $D = 1.5$  compared to  $D = 1.3$ . Because subsurface drain spacing is proportional to the square root of  $\bar{K}_s$  (for example, see drain spacing equations cited by Buckland et al. (1987)), use of  $D = 1.5$  compared to  $D = 1.3$  would result in marginal sacrifice of confidence in drain spacing: that is  $(1.5)^2 \times 100 = 122\%$  versus  $(1.3)^2 \times 100 = 114\%$ , or an 8% difference. Similar comparisons can be done using other  $D$  values.

#### REFERENCES

- BOUWER, H. 1969. Planning and interpreting soil permeability measurements. *J. Irrig. Drain. Div., Proc. Am. Soc. Civil Eng.* 95 (IR3): 391–402.
- BOUWER, H. and R. D. JACKSON. 1974. Determining soil properties. Pages 611–672 in J. van Schilfgaarde (ed.). *Drainage for agriculture*. Agronomy 17, Am. Soc. Agron., Madison, Wis.
- BUCKLAND, G. D., T. G. SOMMERFELDT, and D. B. HARKER. 1987. A field comparison of transient drain spacing equations in a southern Alberta lacustrine soil. *Trans. ASAE (Am. Soc. Agric. Eng.)* 30: 137–142.
- DYLLA, A. S. and J. C. GUITJENS. 1970. Hydraulic conductivity sampling for confidence. *Trans. ASAE (Am. Soc. Agric. Eng.)* 13: 485–488.
- LEE, D. M., W. D. REYNOLDS, D. E. ELRICK, and B. E. CLOTHIER. 1985. A comparison of three field methods for measuring saturated hydraulic conductivity. *Can. J. Soil Sci.* 65: 563–573.
- NIELSEN, D. R., J. W. BIGGAR, and K. T. ERH. 1973. Spatial variability of field-measured soil-water properties. *Hilgardia* 42: 215–260.
- PETERSEN, R. G. and L. D. CALVIN. 1986. Sampling. Pages 33–51 in A. Klute ed. *Methods of soil analysis*. Part 1. 2nd ed. Agronomy 9, Am. Soc. Agron., Madison, Wis.
- STEEL, R. G. D. and J. H. TORRIE. 1980. Principles and procedures of statistics: A biometrical approach. 2nd ed. McGraw-Hill, New York.
- TOPP, G. C., W. D. ZEBCHUK, and J. DUMANSKI. 1980. The variation of in-situ measured soil water properties within soil map units. *Can. J. Soil Sci.* 60: 497–509.
- WARRICK, A. W. and D. R. NIELSEN. 1980. Spatial variability of soil physical properties in the field. Pages 319–344 in D. Hillel, ed. *Applications of soil physics*. Academic Press, New York.
- WILLARDSON, L. S. and R. L. HURST. 1965. Sample size estimates in permeability studies. *J. Irrig. Drain. Div., Proc. Am. Soc. Civil Eng.* 91(IR1): 1–9.